

Chapter 9: At Grade Intersection Capacity and Level of Service

9.1

Given: 2-lane intersection approach; CBD area
 No cross street pedestrians during left turns
 Lane width = 11 ft
 +2% grade
 No parking lanes; No bus stops
 6% heavy vehicles
 Protected phasing
 $P_{LT} = 0.30$

$$N = 2$$

$$f_w = 0.967 \text{ (Table 9-5)}$$

$$f_{HV} = 0.943 \text{ (Table 9-6)}$$

$$f_g = 0.990 \text{ (Table 9-7)}$$

$$f_p = 1.0 \text{ (Table 9-8)}$$

$$f_{bb} = 1.0 \text{ (Table 9-9)}$$

$$f_a = 0.90 \text{ (Table 9-10)}$$

$$f_{RT} = 1 \text{ (Table 9-11a, no right turns)}$$

$$f_{LT} = \frac{1.0}{1.0 + 0.05P_{LT}} = \frac{1.0}{1.0 + 0.05(0.30)} = 0.985 \text{ (Table 9 - 12)}$$

$$s = S_0 N f_w f_{HV} f_g f_p f_{bb} f_a f_{RT} f_{LT}$$

$$s = 1900(2)(0.967)(0.943)(0.990)(1.0)(1.0)(0.90)(1.0)(0.985) = 3042 \text{ vph green time}$$

9.2

The following is the Highway Capacity Software (HCS) solution to the exercise. The specific answers requested in the problem statement are highlighted in bold.

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HCS:  Signalized Intersection      Version 2.4a                08-08-1996  1
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Streets: (E-W) Third Ave.          (N-S) Main St.
Analyst:                            File Name: Exerc_02.HC9
Area Type: CBD                      7-18-96 4-5 PM
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Traffic and Roadway Conditions

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	> 2	<		> 2	<		> 1	<		> 1	<	
Volumes	60	700	50	50	550	20	40	500	30	45	500	15
PHF or PK15	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Lane Width	11.0			11.0			15.0			15.0		
Grade	0			0			0			0		
% Heavy Veh	5	5	5	5	5	5	5	5	5	5	5	5
Parking	(Y/N)	N		(Y/N)	N		(Y/N)	N		(Y/N)	N	
Bus Stops			0			0			0			0

/LnGrp	Flow	Lns	W	HV	G	p	BB	A	RT	LT	Flow
EB											
LTR	1900	2	0.97	0.95	1.00	1.00	1.00	0.90	0.99	0.74	2326
WB											
LTR	1900	2	0.97	0.95	1.00	1.00	1.00	0.90	1.00	0.66	2078
NB											
LTR	1900	1	1.10	0.95	1.00	1.00	1.00	0.90	0.90	0.81	1302
SB											
LTR	1900	1	1.10	0.95	1.00	1.00	1.00	0.90	0.90	0.79	1263

Supplemental Permitted LT Worksheet

APPROACH	EB
Cycle Length, C	70
Actual Green Time for Lane Group, G	27
Effective Green Time for Lane Group, g	27
Opposing Effective Green Time, go	27
Number of Opposing Lanes, No	2
Number of Lanes in Lane Group, N	2
Adjusted Left-Turn Flow Rate, Vlt	67
Proportion of Left Turns in Lane Group, Plt	0.08
Left Turns per Cycle: LTC=Vlt*C/3600	1.30
Adjusted Opposing Flow Rate, Vo	712
Opposing Flow per Lane, Per Cycle: Volc=VoC/3600No	6.92
Opposing Platoon Ratio, Rpo	0.67
Lost time per phase, tl	3
gf=Gexp(-0.882*LTC^0.717)-tl	6.30
Opposing Queue Ratio: qro=1-Rpo(go/C)	0.74
gq = Volc * qro / (.5 - Volc * (1 - qro) / go)-tl	8.83
gu=g-gq (or g-gf)	18.17
fs=(875-0.625Vo)/1000	0.43
Pl=Plt[1+{(N-1)g/(fsgu+4.5)}]	0.24
E11	4.94
fmin	0.09
fm, (min=fmin;max=1.00)	0.58
flt=[fm+0.91(N-1)]/N	0.74

APPROACH	WB
Cycle Length, C	70
Actual Green Time for Lane Group, G	27
Effective Green Time for Lane Group, g	27
Opposing Effective Green Time, go	27
Number of Opposing Lanes, No	2
Number of Lanes in Lane Group, N	2
Adjusted Left-Turn Flow Rate, Vlt	56
Proportion of Left Turns in Lane Group, Plt	0.08
Left Turns per Cycle: LTC=Vlt*C/3600	1.09
Adjusted Opposing Flow Rate, Vo	928
Opposing Flow per Lane, Per Cycle: Volc=VoC/3600No	9.02
Opposing Platoon Ratio, Rpo	1.33
Lost time per phase, tl	3
gf=Gexp(-0.882*LTC^0.717)-tl	7.57
Opposing Queue Ratio: qro=1-Rpo(go/C)	0.49
gq = Volc * qro / (.5 - Volc * (1 - qro) / go)-tl	10.37
gu=g-gq (or g-gf)	16.63

$fs=(875-0.625Vo)/1000$	0.29
$Pl=Plt[1+\{(N-1)g/(fsgu+4.5)\}]$	0.32
E11	12.40
fmin	0.10
$fm, (min=fmin;max=1.00)$	0.41
$flt=[fm+0.91(N-1)]/N$	0.66

APPROACH	NB
Cycle Length, C	70
Actual Green Time for Lane Group, G	37
Effective Green Time for Lane Group, g	37
Opposing Effective Green Time, go	37
Number of Lanes in Lane Group, N	1
Proportion of Left Turns in Lane Group, Plt	0.07
Proportion of Left Turns in Opposing Flow, Plto	0.08
Adjusted Left-Turn Flow Rate, Vlt	44
Left Turns per Cycle: $LTC=Vlt*C/3600$	0.86
Adjusted Opposing Flow Rate, Vo	612
Opposing Flow per Lane, Per Cycle: $Volc=VoC/3600No$	11.90
Opposing Platoon Ratio, Rpo	1
Lost time per phase, tl	3
$gf=Gexp(-0.860*LTC^{0.629})-tl$	13.97
Opposing Queue Ratio: $qro=1-Rpo(go/C)$	0.47
$gq=4.943volc^{0.762}*qro^{1.061}-tl$	11.69
$gu=g-gq$ (or $g-gf$)	23.03
$n=(gq-gf)/2$	0.00
$Ptho=1-Plto$	0.92
E12	0.00
E11	7.07
$fmin=2(1+Pl)/g$	0.06
$flt=fm, (min=fmin; max=1)$	0.81

APPROACH	SB
Cycle Length, C	70
Actual Green Time for Lane Group, G	37
Effective Green Time for Lane Group, g	37
Opposing Effective Green Time, go	37
Number of Lanes in Lane Group, N	1
Proportion of Left Turns in Lane Group, Plt	0.08
Proportion of Left Turns in Opposing Flow, Plto	0.07
Adjusted Left-Turn Flow Rate, Vlt	50
Left Turns per Cycle: $LTC=Vlt*C/3600$	0.97
Adjusted Opposing Flow Rate, Vo	611
Opposing Flow per Lane, Per Cycle: $Volc=VoC/3600No$	11.88
Opposing Platoon Ratio, Rpo	1
Lost time per phase, tl	3
$gf=Gexp(-0.860*LTC^{0.629})-tl$	12.90
Opposing Queue Ratio: $qro=1-Rpo(go/C)$	0.47
$gq=4.943volc^{0.762}*qro^{1.061}-tl$	11.67
$gu=g-gq$ (or $g-gf$)	24.10
$n=(gq-gf)/2$	0.00
$Ptho=1-Plto$	0.93
E12	0.00
E11	7.02
$fmin=2(1+Pl)/g$	0.06
$flt=fm, (min=fmin; max=1)$	0.79

Capacity Analysis Worksheet

Direction /LnGrp	Adj Flow Rate (v)	Adj Sat Flow Rate (s)	Flow Ratio (v/s)	Green Ratio (g/C)	Lane Group Capacity (c)	v/c Ratio
EB						
LTR	928	2326	0.399	0.386	897	1.034 *
WB						
LTR	712	2078	0.343	0.386	802	0.888
NB						
LTR	611	1302	0.469	0.529	688	0.888
SB						
LTR	612	1263	0.485	0.529	668	0.917 *
Lost Time/Cycle, L =				6.0 sec	Sum (v/s) critical = 0.884	Critical v/c(x) = 0.966

Level of Service Worksheet

Direction /LnGrp	v/c Ratio	g/C Ratio	Delay d 1	Del Adj Fact	Lane Group Cap	Calib d 2	Delay d 2	Lane Grp Del	Lane Grp LOS	Delay By App	LOS By App
EB											
LTR	1.034	0.386	16.3	0.910	897	12	29.0	43.9	E	43.9	E
WB											
LTR	0.888	0.386	15.3	1.124	802	12	6.7	23.8	C	23.8	C
NB											
LTR	0.888	0.529	11.1	1.000	688	16	9.6	20.7	C	20.7	C
SB											
LTR	0.917	0.529	11.5	1.000	668	16	12.6	24.1	C	24.1	C
Intersection Delay =						29.7 sec/veh	Intersection LOS = D				

9.3

A complete operational analysis based on the Highway Capacity Software (HCS) is included here. Exercise 3 requests the left-turn adjustment factor only for the EB approach, in a case where there is an exclusive left-turn lane. The left-turn adjustment-factor computation is highlighted in bold.

HCS: Signalized Intersection Version 2.4a 08-02-1996 1

Streets: (E-W) Ryder St. (N-S) Moss Ave.
 Analyst: File Name: Exerc_03.HC9
 Area Type: Other 8-1-96

Traffic and Roadway Conditions

| Eastbound | Westbound | Northbound | Southbound

	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	1	2	<	1	2	<	>	1	<	>	1	<
Volumes	80	870	50	80	756	53	35	425	25	15	500	30
PHF or PK15	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Lane Width	12.0	12.0		12.0	12.0		12.0			12.0		
Grade		3			-3			3			-3	
% Heavy Veh	5	5	5	5	5	5	5	5	5	5	5	5
Parking	(Y/N)	N		(Y/N)	N		(Y/N)	N		(Y/N)	N	
Bus Stops			0			0			0			0
Con. Peds			0			0			0			0
Ped Button	(Y/N)	N		(Y/N)	N		(Y/N)	N		(Y/N)	N	
Arr Type	3	3		3	3			3			3	
RTOR Vols			0			0			0			0
Lost Time	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00

Signal Operations

Phase Combination	1	2	3	4	5	6	7	8
EB Left	*				NB Left	*		
Thru	*				Thru	*		
Right	*				Right	*		
Peds					Peds			
WB Left	*				SB Left	*		
Thru	*				Thru	*		
Right	*				Right	*		
Peds					Peds			
NB Right					EB Right			
SB Right					WB Right			
Green	35.0P				Green	27.0P		
Yellow/AR	4.0				Yellow/AR	4.0		

Cycle Length: 70 sec Phase combination order: #1 #5

Volume Adjustment Worksheet

Direction/Mvt	Mvt Vol	PHF	Adj Vol	Lane Grp	Lane Grp Vol	Lane No. Ln	Lane Util Fact	Growth Fact	Adj Grp Vol	Prop LT	Prop RT
EB											
Left	80	0.95	84	L	84	1	1.000	1.000	84	1.00	0.00
Thru	870	0.95	916	TR	969	2	1.050	1.000	1017	0.00	0.05
Right	50	0.95	53								
WB											
Left	80	0.95	84	L	84	1	1.000	1.000	84	1.00	0.00

Thru	756	0.95	796	TR	852	2	1.050	1.000	895	0.00	0.07
Right	53	0.95	56								
NB											
Left	35	0.95	37								
Thru	425	0.95	447	LTR	510	1	1.000	1.000	510	0.07	0.05
Right	25	0.95	26								
SB											
Left	15	0.95	16								
Thru	500	0.95	526	LTR	574	1	1.000	1.000	574	0.03	0.06
Right	30	0.95	32								

Saturation Flow Adjustment Worksheet

Direction /LnGrp	Ideal										Adj Sat Flow	
	Sat Flow	No. Lns	f W	f HV	f G	f p	f BB	f A	f RT	f LT		

EB												
L	1900	1	1.00	0.95	0.99	1.00	1.00	1.00	1.00	1.00	0.15	270
TR	1900	2	1.00	0.95	0.99	1.00	1.00	1.00	1.00	0.99	1.00	3538
WB												
L	1900	1	1.00	0.95	1.01	1.00	1.00	1.00	1.00	1.00	0.11	204
TR	1900	2	1.00	0.95	1.01	1.00	1.00	1.00	1.00	0.99	1.00	3635
NB												
LTR	1900	1	1.00	0.95	0.99	1.00	1.00	1.00	1.00	0.89	0.86	1372
SB												
LTR	1900	1	1.00	0.95	1.01	1.00	1.00	1.00	1.00	0.89	0.95	1560

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Supplemental Permitted LT Worksheet

APPROACH	EB
Cycle Length, C	70
Actual Green Time for Lane Group, G	35
Effective Green Time for Lane Group, g	36
Opposing Effective Green Time, go	36
Number of Opposing Lanes, No	2
Number of Lanes in Lane Group, N	1
Adjusted Left-Turn Flow Rate, Vlt	84
Proportion of Left Turns in Lane Group, Plt	1.00
Left Turns per Cycle: LTC=Vlt*C/3600	1.63
Adjusted Opposing Flow Rate, Vo	895
Opposing Flow per Lane, Per Cycle: Volc=VoC/3600No	8.70
Opposing Platoon Ratio, Rpo	1
Lost time per phase, t1	3
gf=Gexp(-0.882*LTC^0.717)-t1	0.00
Opposing Queue Ratio: qro=1-Rpo(go/C)	0.49
gq = Volc * qro / (.5 - Volc * (1 - qro) / go)-t1	8.25
gu=g-gq (or g-gf)	27.75
fs=(875-0.625Vo)/1000	0.32
Pl=Plt[1+{(N-1)g/(fsgu+4.5)}]	1.00
E11	5.10
fmin	0.11
fm, (min=fmin;max=1.00)	0.15
flt=[fm+0.91(N-1)]/N	0.15

APPROACH	WB
Cycle Length, C	70
Actual Green Time for Lane Group, G	35
Effective Green Time for Lane Group, g	36
Opposing Effective Green Time, go	36
Number of Opposing Lanes, No	2
Number of Lanes in Lane Group, N	1
Adjusted Left-Turn Flow Rate, Vlt	84
Proportion of Left Turns in Lane Group, Plt	1.00
Left Turns per Cycle: $LTC=Vlt*C/3600$	1.63
Adjusted Opposing Flow Rate, Vo	1017
Opposing Flow per Lane, Per Cycle: $Volc=VoC/3600No$	9.89
Opposing Platoon Ratio, Rpo	1
Lost time per phase, tl	3
$gf=Gexp(-0.882*LTC^0.717)-tl$	0.00
Opposing Queue Ratio: $qro=1-Rpo(go/C)$	0.49
$gq = Volc * qro / (.5 - Volc * (1 - qro) / go)-tl$	10.39
$gu=g-gq$ (or $g-gf$)	25.61
$fs=(875-0.625Vo)/1000$	0.24
$Pl=Plt[1+{(N-1)g/(fsgu+4.5)}]$	1.00
E11	6.56
fmin	0.11
$fm, (min=fmin;max=1.00)$	0.11
$flt=[fm+0.91(N-1)]/N$	0.11

APPROACH	NB
Cycle Length, C	70
Actual Green Time for Lane Group, G	27
Effective Green Time for Lane Group, g	28
Opposing Effective Green Time, go	28
Number of Lanes in Lane Group, N	1
Proportion of Left Turns in Lane Group, Plt	0.07
Proportion of Left Turns in Opposing Flow, Plto	0.03
Adjusted Left-Turn Flow Rate, Vlt	37
Left Turns per Cycle: $LTC=Vlt*C/3600$	0.72
Adjusted Opposing Flow Rate, Vo	574
Opposing Flow per Lane, Per Cycle: $Volc=VoC/3600No$	11.16
Opposing Platoon Ratio, Rpo	1
Lost time per phase, tl	3
$gf=Gexp(-0.860*LTC^0.629)-tl$	10.42
Opposing Queue Ratio: $qro=1-Rpo(go/C)$	0.60
$gq=4.943volc^0.762*qro^1.061-tl$	15.07
$gu=g-gq$ (or $g-gf$)	12.93
$n=(gq-gf)/2$	2.33
$Ptho=1-Plto$	0.97
E12	2.28
E11	6.08
$fmin=2(1+Pl)/g$	0.08
$flt=fm, (min=fmin; max=1)$	0.86

APPROACH	SB
Cycle Length, C	70
Actual Green Time for Lane Group, G	27
Effective Green Time for Lane Group, g	28
Opposing Effective Green Time, go	28
Number of Lanes in Lane Group, N	1

Proportion of Left Turns in Lane Group, Plt 0.03
 Proportion of Left Turns in Opposing Flow, Plto 0.07
 Adjusted Left-Turn Flow Rate, Vlt 16
 Left Turns per Cycle: LTC=Vlt*C/3600 0.31
 Adjusted Opposing Flow Rate, Vo 510
 Opposing Flow per Lane, Per Cycle: Volc=VoC/3600No 9.92
 Opposing Platoon Ratio, Rpo 1
 Lost time per phase, t1 3
 $gf=Gexp(-0.860*LTC^{0.629})-t1$ 14.87
 Opposing Queue Ratio: qro=1-Rpo(go/C) 0.60
 $gq=4.943volc^{0.762}*qro^{1.061}-t1$ 13.51
 $gu=g-gq$ (or $g-gf$) 13.13
 $n=(gq-gf)/2$ 0.00
 $Ptho=1-Plto$ 0.93
 El2 0.00
 El1 5.06
 $fmin=2(1+Pl)/g$ 0.07
 $flt=fm, (min=fmin; max=1)$ 0.95

Capacity Analysis Worksheet

Direction /LnGrp	Adj Flow Rate (v)	Adj Sat Flow Rate (s)	Flow Ratio (v/s)	Green Ratio (g/C)	Lane Group Capacity (c)	v/c Ratio
EB						
L	84	270	0.311	0.514	139	0.605
TR	1017	3538	0.287	0.514	1820	0.559
WB						
L	84	204	0.412	0.514	105	0.801 *
TR	895	3635	0.246	0.514	1869	0.479
NB						
LTR	510	1372	0.372	0.400	549	0.929 *
SB						
LTR	574	1560	0.368	0.400	624	0.920
			Sum (v/s) critical = 0.783			
Lost Time/Cycle, L =		6.0 sec	Critical v/c(x)		= 0.857	

Level of Service Worksheet

Direction /LnGrp	v/c Ratio	g/C Ratio	Delay d 1	Del Adj Fact	Lane Group Cap	Calib d 2	Delay d 2	Lane Grp Del	Lane Grp LOS	Delay By App	LOS By App
EB											
L	0.605	0.514	9.1	1.000	139	16	5.1	14.2	B	9.5	B
TR	0.559	0.514	8.8	1.000	1820	16	0.3	9.1	B		
WB											
L	0.801	0.514	10.7	1.000	105	16	22.5	33.2	D	10.6	B
TR	0.479	0.514	8.3	1.000	1869	16	0.2	8.5	B		
NB											
LTR	0.929	0.400	15.2	1.000	549	16	16.2	31.4	D	31.4	D
SB											
LTR	0.920	0.400	15.2	1.000	624	16	13.6	28.8	D	28.8	D

Intersection Delay = 16.9 sec/veh Intersection LOS = C

9.4

The following is the Highway Capacity Software (HCS) solution to Exercise 9. Exercise 4 requests the left-turn adjustment factor only for the EB approach, in a case where the left turns are made from a shared lane. Computation of the left-turn adjustment factor is highlighted in bold.

HCS: Signalized Intersection Version 2.4a 08-02-1996 1

Streets: (E-W) Ryder St. (N-S) Moss Ave.
 Analyst: File Name: Exer_4&9.HC9
 Area Type: Other 8-1-96

Traffic and Roadway Conditions

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	> 2	<		> 2	<		> 1	<		> 1	<	
Volumes	80	870	50	80	756	53	35	425	25	15	500	30
PHF or PK15	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Lane Width	12.0			12.0			12.0			12.0		
Grade	3			-3			3			-3		
% Heavy Veh	5	5	5	5	5	5	5	5	5	5	5	5
Parking	(Y/N)	N		(Y/N)	N		(Y/N)	N		(Y/N)	N	
Bus Stops			0			0			0			0
Con. Peds			0			0			0			0
Ped Button	(Y/N)	N		(Y/N)	N		(Y/N)	N		(Y/N)	N	
Arr Type		3			3			3			3	
RTOR Vols			0			0			0			0
Lost Time	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00

Signal Operations

Phase Combination	1	2	3	4	5	6	7	8
EB Left	*				NB Left	*		
Thru	*				Thru	*		
Right	*				Right	*		
Peds					Peds			
WB Left		*			SB Left		*	
Thru		*			Thru		*	
Right		*			Right		*	
Peds					Peds			
NB Right					EB Right			
SB Right					WB Right			
Green	35.0P				Green	27.0P		
Yellow/AR	4.0				Yellow/AR	4.0		

Cycle Length: 70 sec Phase combination order: #1 #5

Volume Adjustment Worksheet

Direction/ Mvt	Mvt Vol	PHF	Adj Vol	Lane Grp	Lane Grp Vol	Lane No. Ln	Lane Util Fact	Growth Fact	Adj Grp Vol	Prop LT	Prop RT
EB											
Left	80	0.95	84								
Thru	870	0.95	916	LTR	1053	2	1.050	1.000	1106	0.08	0.05
Right	50	0.95	53								
WB											
Left	80	0.95	84								
Thru	756	0.95	796	LTR	936	2	1.050	1.000	983	0.09	0.06
Right	53	0.95	56								
NB											
Left	35	0.95	37								
Thru	425	0.95	447	LTR	510	1	1.000	1.000	510	0.07	0.05
Right	25	0.95	26								
SB											
Left	15	0.95	16								
Thru	500	0.95	526	LTR	574	1	1.000	1.000	574	0.03	0.06
Right	30	0.95	32								

Saturation Flow Adjustment Worksheet

Direction /LnGrp	Ideal Sat Flow	No. Lns	f W	f HV	f G	f p	f BB	f A	f RT	f LT	Adj Sat Flow
EB											
LTR	1900	2	1.00	0.95	0.99	1.00	1.00	1.00	0.99	0.62	2179
WB											
LTR	1900	2	1.00	0.95	1.01	1.00	1.00	1.00	0.99	0.59	2165
NB											
LTR	1900	1	1.00	0.95	0.99	1.00	1.00	1.00	0.89	0.86	1372
SB											
LTR	1900	1	1.00	0.95	1.01	1.00	1.00	1.00	0.89	0.95	1560

Supplemental Permitted LT Worksheet

APPROACH	EB
Cycle Length, C	70
Actual Green Time for Lane Group, G	35
Effective Green Time for Lane Group, g	36
Opposing Effective Green Time, go	36

Number of Opposing Lanes, No	2
Number of Lanes in Lane Group, N	2
Adjusted Left-Turn Flow Rate, Vlt	84
Proportion of Left Turns in Lane Group, Plt	0.08
Left Turns per Cycle: LTC=Vlt*C/3600	1.63
Adjusted Opposing Flow Rate, Vo	983
Opposing Flow per Lane, Per Cycle: Volc=VoC/3600No	9.56
Opposing Platoon Ratio, Rpo	1
Lost time per phase, tl	3
gf=Gexp(-0.882*LTC^0.717)-tl	6.99
Opposing Queue Ratio: qro=1-Rpo(go/C)	0.49
gq = Volc * qro / (.5 - Volc * (1 - qro) / go)-tl	9.77
gu=g-gq (or g-gf)	26.23
fs=(875-0.625Vo)/1000	0.26
Pl=Plt[1+{(N-1)g/(fsgu+4.5)}]	0.33
E11	15.15
fmin	0.07
fm, (min=fmin;max=1.00)	0.32
flt=[fm+0.91(N-1)]/N	0.62

APPROACH	WB
Cycle Length, C	70
Actual Green Time for Lane Group, G	35
Effective Green Time for Lane Group, g	36
Opposing Effective Green Time, go	36
Number of Opposing Lanes, No	2
Number of Lanes in Lane Group, N	2
Adjusted Left-Turn Flow Rate, Vlt	84
Proportion of Left Turns in Lane Group, Plt	0.09
Left Turns per Cycle: LTC=Vlt*C/3600	1.63
Adjusted Opposing Flow Rate, Vo	1106
Opposing Flow per Lane, Per Cycle: Volc=VoC/3600No	10.75
Opposing Platoon Ratio, Rpo	1
Lost time per phase, tl	3
gf=Gexp(-0.882*LTC^0.717)-tl	6.99
Opposing Queue Ratio: qro=1-Rpo(go/C)	0.49
gq = Volc * qro / (.5 - Volc * (1 - qro) / go)-tl	12.08
gu=g-gq (or g-gf)	23.92
fs=(875-0.625Vo)/1000	0.18
Pl=Plt[1+{(N-1)g/(fsgu+4.5)}]	0.45
E11	16.00
fmin	0.08
fm, (min=fmin;max=1.00)	0.28
flt=[fm+0.91(N-1)]/N	0.59

APPROACH	NB
Cycle Length, C	70
Actual Green Time for Lane Group, G	27
Effective Green Time for Lane Group, g	28
Opposing Effective Green Time, go	28
Number of Lanes in Lane Group, N	1
Proportion of Left Turns in Lane Group, Plt	0.07
Proportion of Left Turns in Opposing Flow, Plto	0.03
Adjusted Left-Turn Flow Rate, Vlt	37
Left Turns per Cycle: LTC=Vlt*C/3600	0.72
Adjusted Opposing Flow Rate, Vo	574

Opposing Flow per Lane, Per Cycle: $Volc=VoC/3600No$	11.16
Opposing Platoon Ratio, Rpo	1
Lost time per phase, t_l	3
$gf=Gexp(-0.860*LTC^{0.629})-t_l$	10.42
Opposing Queue Ratio: $qro=1-Rpo(go/C)$	0.60
$gq=4.943volc^{0.762}*qro^{1.061}-t_l$	15.07
$gu=g-gq$ (or $g-gf$)	12.93
$n=(gq-gf)/2$	2.33
$Ptho=1-Plto$	0.97
E12	2.28
E11	6.08
$fmin=2(1+Pl)/g$	0.08
$flt=fm, (min=fmin; max=1)$	0.86

APPROACH	SB
Cycle Length, C	70
Actual Green Time for Lane Group, G	27
Effective Green Time for Lane Group, g	28
Opposing Effective Green Time, go	28
Number of Lanes in Lane Group, N	1
Proportion of Left Turns in Lane Group, Plt	0.03
Proportion of Left Turns in Opposing Flow, $Plto$	0.07
Adjusted Left-Turn Flow Rate, Vlt	16
Left Turns per Cycle: $LTC=Vlt*C/3600$	0.31
Adjusted Opposing Flow Rate, Vo	510
Opposing Flow per Lane, Per Cycle: $Volc=VoC/3600No$	9.92
Opposing Platoon Ratio, Rpo	1
Lost time per phase, t_l	3
$gf=Gexp(-0.860*LTC^{0.629})-t_l$	14.87
Opposing Queue Ratio: $qro=1-Rpo(go/C)$	0.60
$gq=4.943volc^{0.762}*qro^{1.061}-t_l$	13.51
$gu=g-gq$ (or $g-gf$)	13.13
$n=(gq-gf)/2$	0.00
$Ptho=1-Plto$	0.93
E12	0.00
E11	5.06
$fmin=2(1+Pl)/g$	0.07
$flt=fm, (min=fmin; max=1)$	0.95

=====
Capacity Analysis Worksheet
=====

Direction / LnGrp	Adj Flow Rate (v)	Adj Sat Flow Rate (s)	Flow Ratio (v/s)	Green Ratio (g/C)	Lane Group Capacity (c)	v/c Ratio
EB						
LTR	1106	2179	0.508	0.514	1121	0.987 *
WB						
LTR	983	2165	0.454	0.514	1113	0.883
NB						
LTR	510	1372	0.372	0.400	549	0.929 *
SB						
LTR	574	1560	0.368	0.400	624	0.920
Lost Time/Cycle, L = 6.0 sec				Sum (v/s) critical = 0.879		
				Critical v/c(x) = 0.962		

 Level of Service Worksheet

Direction /LnGrp	v/c Ratio	g/C Ratio	Delay d 1	Del Adj Fact	Lane Group Cap	Calib d 2	Delay d 2	Lane Grp Del	Lane Grp LOS	Delay By App	LOS By App
EB											
LTR	0.987	0.514	12.7	1.000	1121	16	17.9	30.7	D	30.7	D
WB											
LTR	0.883	0.514	11.5	1.000	1113	16	6.1	17.6	C	17.6	C
NB											
LTR	0.929	0.400	15.2	1.000	549	16	16.2	31.4	D	31.4	D
SB											
LTR	0.920	0.400	15.2	1.000	624	16	13.6	28.8	D	28.8	D
			Intersection Delay = 26.4 sec/veh Intersection LOS = D								

9.5

The following is the Highway Capacity Software (HCS) solution to the exercise. The specific answers requested in the problem statement are highlighted in bold.

```

HCS:   Signalized Intersection      Version 2.4a                08-08-1996  1
=====
Streets: (E-W) Grand Ave.          (N-S) 4th St.
Analyst:                               File Name: Exerc_05.HC9
Area Type: CBD                        7-30-96
=====
    
```

Traffic and Roadway Conditions

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	> 2	<		> 2	<		> 1	<		> 1	<	
Volumes	60	570	30	25	635	18	27	330	20	35	450	45
PHF or PK15	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Lane Width	12.0			12.0			14.0			14.0		
Grade	0			0			0			0		
% Heavy Veh	5	5	5	5	5	5	5	5	5	5	5	5
Parking	(Y/N)	N		(Y/N)	N		(Y/N)	N		(Y/N)	N	
Bus Stops			0			0			0			0
Con. Peds			80			80			80			80
Ped Button	(Y/N)	N		(Y/N)	N		(Y/N)	N		(Y/N)	N	
Arr Type		4			2			3			3	
RTOR Vols			0			0			0			0

Lost Time |3.00 3.00 3.00|3.00 3.00 3.00|3.00 3.00 3.00|3.00 3.00 3.00

Signal Operations

```

-----
Phase Combination 1      2      3      4 |      5      6      7      8
EB  Left             *                |NB  Left             *
    Thru             *                |  Thru             *
    Right            *                |  Right            *
    Peds              |  Peds
                               |
WB  Left             *                |SB  Left             *
    Thru             *                |  Thru             *
    Right            *                |  Right            *
    Peds              |  Peds
                               |
NB  Right            |EB  Right
SB  Right            |WB  Right
                               |
Green                30.0P           |Green                27.0P
Yellow/AR            4.0             |Yellow/AR            4.0
-----

```

Cycle Length: 65 sec Phase combination order: #1 #5

Volume Adjustment Worksheet

```

-----
Direc-      Lane      Lane      Adj
tion/ Mvt      Adj      Lane      Grp      No.      Util      Growth      Grp      Prop      Prop
Mvt  Vol      PHF      Vol      Grp      Vol      Ln      Fact      Fact      Vol      LT      RT
-----
EB
Left      60      0.85      71
Thru     570      0.85      671      LTR      777      2      1.050      1.000      816      0.09      0.05
Right     30      0.85      35
WB
Left      25      0.85      29
Thru     635      0.85      747      LTR      797      2      1.050      1.000      837      0.04      0.03
Right     18      0.85      21
NB
Left      27      0.85      32
Thru     330      0.85      388      LTR      444      1      1.000      1.000      444      0.07      0.05
Right     20      0.85      24
SB
Left      35      0.85      41
Thru     450      0.85      529      LTR      623      1      1.000      1.000      623      0.07      0.09
Right     45      0.85      53
-----

```

Saturation Flow Adjustment Worksheet

```

-----
                Ideal
Direction Sat  No.  f    f    f    f    f    f    f    f    f    Adj
/LnGrp   Flow  Lns  W    HV   G    p    BB   A    RT   LT   Sat
-----
EB
-----

```

	LTR	1900	2	1.00	0.95	1.00	1.00	1.00	0.90	0.99	0.68	2215
WB	LTR	1900	2	1.00	0.95	1.00	1.00	1.00	0.90	1.00	0.83	2701
NB	LTR	1900	1	1.07	0.95	1.00	1.00	1.00	0.90	0.89	0.84	1299
SB	LTR	1900	1	1.07	0.95	1.00	1.00	1.00	0.90	0.89	0.90	1377

=====

Supplemental Permitted LT Worksheet

APPROACH	EB
Cycle Length, C	65
Actual Green Time for Lane Group, G	30
Effective Green Time for Lane Group, g	31
Opposing Effective Green Time, go	31
Number of Opposing Lanes, No	2
Number of Lanes in Lane Group, N	2
Adjusted Left-Turn Flow Rate, Vlt	71
Proportion of Left Turns in Lane Group, Plt	0.09
Left Turns per Cycle: LTC=Vlt*C/3600	1.28
Adjusted Opposing Flow Rate, Vo	837
Opposing Flow per Lane, Per Cycle: Volc=VoC/3600No	7.56
Opposing Platoon Ratio, Rpo	0.67
Lost time per phase, tl	3
gf=Gexp(-0.882*LTC^0.717)-tl	7.46
Opposing Queue Ratio: qro=1-Rpo(go/C)	0.68
gq = Volc * qro / (.5 - Volc * (1 - qro) / go)-tl	9.18
gu=g-gq (or g-gf)	21.82
fs=(875-0.625Vo)/1000	0.35
Pl=Plt[1+{(N-1)g/(fsgu+4.5)}]	0.32
E11	7.85
fmin	0.09
fm, (min=fmin;max=1.00)	0.46
flt=[fm+0.91(N-1)]/N	0.68

APPROACH	WB
Cycle Length, C	65
Actual Green Time for Lane Group, G	30
Effective Green Time for Lane Group, g	31
Opposing Effective Green Time, go	31
Number of Opposing Lanes, No	2
Number of Lanes in Lane Group, N	2
Adjusted Left-Turn Flow Rate, Vlt	29
Proportion of Left Turns in Lane Group, Plt	0.04
Left Turns per Cycle: LTC=Vlt*C/3600	0.52
Adjusted Opposing Flow Rate, Vo	816
Opposing Flow per Lane, Per Cycle: Volc=VoC/3600No	7.37
Opposing Platoon Ratio, Rpo	1.33
Lost time per phase, tl	3
gf=Gexp(-0.882*LTC^0.717)-tl	14.23
Opposing Queue Ratio: qro=1-Rpo(go/C)	0.37
gq = Volc * qro / (.5 - Volc * (1 - qro) / go)-tl	4.71
gu=g-gq (or g-gf)	16.77
fs=(875-0.625Vo)/1000	0.37
Pl=Plt[1+{(N-1)g/(fsgu+4.5)}]	0.14
E11	6.80

fmin	0.07
fm, (min=fmin;max=1.00)	0.76
flt=[fm+0.91(N-1)]/N	0.83

APPROACH	NB
Cycle Length, C	65
Actual Green Time for Lane Group, G	27
Effective Green Time for Lane Group, g	28
Opposing Effective Green Time, go	28
Number of Lanes in Lane Group, N	1
Proportion of Left Turns in Lane Group, Plt	0.07
Proportion of Left Turns in Opposing Flow, Plto	0.07
Adjusted Left-Turn Flow Rate, Vlt	32
Left Turns per Cycle: LTC=Vlt*C/3600	0.58
Adjusted Opposing Flow Rate, Vo	623
Opposing Flow per Lane, Per Cycle: Volc=VoC/3600No	11.25
Opposing Platoon Ratio, Rpo	1
Lost time per phase, tl	3
gf=Gexp(-0.860*LTC^0.629)-tl	11.68
Opposing Queue Ratio: qro=1-Rpo(go/C)	0.57
gq=4.943volc^0.762*qro^1.061-tl	14.19
gu=g-gq (or g-gf)	13.81
n=(gq-gf)/2	1.25
Ptho=1-Plto	0.93
E12	1.24
E11	7.59
fmin=2(1+Pl)/g	0.08
flt=fm, (min=fmin; max=1)	0.84

APPROACH	SB
Cycle Length, C	65
Actual Green Time for Lane Group, G	27
Effective Green Time for Lane Group, g	28
Opposing Effective Green Time, go	28
Number of Lanes in Lane Group, N	1
Proportion of Left Turns in Lane Group, Plt	0.07
Proportion of Left Turns in Opposing Flow, Plto	0.07
Adjusted Left-Turn Flow Rate, Vlt	41
Left Turns per Cycle: LTC=Vlt*C/3600	0.74
Adjusted Opposing Flow Rate, Vo	444
Opposing Flow per Lane, Per Cycle: Volc=VoC/3600No	8.02
Opposing Platoon Ratio, Rpo	1
Lost time per phase, tl	3
gf=Gexp(-0.860*LTC^0.629)-tl	10.25
Opposing Queue Ratio: qro=1-Rpo(go/C)	0.57
gq=4.943volc^0.762*qro^1.061-tl	10.28
gu=g-gq (or g-gf)	17.72
n=(gq-gf)/2	0.01
Ptho=1-Plto	0.93
E12	0.02
E11	4.00
fmin=2(1+Pl)/g	0.08
flt=fm, (min=fmin; max=1)	0.90

Direction /LnGrp	Adj Flow Rate (v)	Adj Sat Flow Rate (s)	Flow Ratio (v/s)	Green Ratio (g/C)	Lane Group Capacity (c)	v/c Ratio
EB						
LTR	816	2215	0.368	0.477	1056	0.772 *
WB						
LTR	837	2701	0.310	0.477	1288	0.650
NB						
LTR	444	1299	0.342	0.431	560	0.793
SB						
LTR	623	1377	0.452	0.431	593	1.050 *
Lost Time/Cycle, L =			6.0 sec		Sum (v/s) critical = 0.821	
			Critical v/c(x)		= 0.904	

Level of Service Worksheet

Direction /LnGrp	v/c Ratio	g/C Ratio	Delay d 1	Del Adj Fact	Lane Group Cap	Calib d 2	Delay d 2	Lane Grp Del	Lane Grp LOS	Delay By App	LOS By App
EB											
LTR	0.772	0.477	10.7	0.801	1056	12	1.9	10.5	B	10.5	B
WB											
LTR	0.650	0.477	9.8	1.212	1288	12	0.6	12.5	B	12.5	B
NB											
LTR	0.793	0.431	12.2	1.000	560	16	5.3	17.5	C	17.5	C
SB											
LTR	1.050	0.431	14.1	1.000	593	16	43.1	57.2	E	57.2	E
Intersection Delay =						22.9 sec/veh	Intersection LOS = C				

Even though the SB approach is at capacity, the overall intersection level of service is C. Therefore, the existing design is acceptable.

9.6

The following is the Highway Capacity Software (HCS) solution to the exercise. This is one suitable three-phase signal plan. This plan is acceptable without any improvements to the given geometry.

```

HCS:   Signalized Intersection      Version 2.4a                08-08-1996  1
=====
Streets: (E-W) Pine Blvd.          (N-S) Oak St.
Analyst:                            File Name: Exerc_06.HC9
Area Type: Other                    7-30-96
=====

```


SB

Saturation Flow Adjustment Worksheet

Direction /LnGrp	Ideal Sat Flow	No. Lns	f W	f HV	f G	f p	f BB	f A	f RT	f LT	Adj Sat Flow
EB										0.12	204
L	1900	1	1.00	0.91	1.00	1.00	1.00	1.00	1.00	0.95	1649
T	1900	2	1.00	0.91	1.00	1.00	0.98	1.00	1.00	1.00	3402
WB											
TR	1900	2	1.00	0.91	1.00	1.00	0.98	1.00	0.98	1.00	3301
NB											
LTR	1900	2	1.00	0.95	1.00	0.93	1.00	1.00	1.00	1.00	3324
SB											

Supplemental Permitted LT Worksheet

APPROACH	EB
Cycle Length, C	78
Actual Green Time for Lane Group, G	44
Effective Green Time for Lane Group, g	34
Opposing Effective Green Time, go	31
Number of Opposing Lanes, No	2
Number of Lanes in Lane Group, N	1
Adjusted Left-Turn Flow Rate, Vlt	105
Proportion of Left Turns in Lane Group, Plt	1.00
Left Turns per Cycle: LTC=Vlt*C/3600	2.27
Adjusted Opposing Flow Rate, Vo	828
Opposing Flow per Lane, Per Cycle: Volc=VoC/3600No	8.97
Opposing Platoon Ratio, Rpo	1
Lost time per phase, tl	0
gf=Gexp(-0.882*LTC^0.717)-tl	0.00
Opposing Queue Ratio: qro=1-Rpo(go/C)	0.60
gq = Volc * qro / (.5 - Volc * (1 - qro) / go)-tl	14.04
gu=g-gq (or g-gf)	19.96
fs=(875-0.625Vo)/1000	0.36
Pl=Plt[1+{(N-1)g/(fsgu+4.5)}]	1.00
E11	6.22
fmin	0.12
fm, (min=fmin;max=1.00)	0.12
flt=[fm+0.91(N-1)]/N	0.12

Capacity Analysis Worksheet

Direction /LnGrp	Adj Flow Rate (v)	Adj Sat Flow Rate (s)	Flow Ratio (v/s)	Green Ratio (g/C)	Lane Group Capacity (c)	v/c Ratio
EB Lsec.	0	204	0.000	0.436	92	0.000
Lpri.	105	1649	0.064	0.141	233	0.451 *
Ltot.	105				325	0.323

T	994	3402	0.292	0.577	1963	0.506
WB						
TR	828	3301	0.251	0.397	1312	0.631 *
NB						
LTR	840	3324	0.253	0.346	1151	0.730 *
SB						
			Sum (v/s) critical = 0.567			
Lost Time/Cycle, L = 9.0 sec			Critical v/c(x) = 0.641			

Level of Service Worksheet

Direction /LnGrp	v/c Ratio	g/C Ratio	Delay d 1	Del Adj Fact	Lane Group Cap	Calib d 2	Delay d 2	Lane Grp Del	Lane Grp LOS	Delay By App	LOS By App
EB											
L	0.323	0.321	7.0	1.000	325	16	0.2	7.3	B	7.6	B
T	0.506	0.577	7.5	1.000	1963	16	0.2	7.7	B		
WB											
TR	0.631	0.397	14.4	1.000	1312	16	0.7	15.1	C	15.1	C
NB											
LTR	0.730	0.346	17.0	1.000	1151	16	1.7	18.6	C	18.6	C
SB											
			Intersection Delay = 13.2 sec/veh Intersection LOS = B								

Supplemental Uniform Delay Worksheet

Approach	Eastbound
Adj. LT Vol (v)	105
v/c ratio (x)	0.32
Primary phase effective green	11.00
gq from Supplemental LT Worksheet	14.04
gu from Supplemental LT Worksheet	19.96
Red time (r)	33.00
Arrivals qa = v/(3600(max(x,1))	0.03
Primary Ph. Departures Sp=s/3600	0.46
Secondary Ph. Departures Ss=S(Gq+Gu)/(Gu*3600)	0.10
Xperm	0.51
Xprot	0.25
Case	1
Queue at beginning of green arrow (Qa)	0.96
Queue at beginning of unsaturated green (Qu)	0.41
Residual queue (Qr)	0.00
Uniform Delay	7.04

9.7

The following is the Highway Capacity Software (HCS) solution to the exercise.

Streets: (E-W) Main St.
 Analyst:
 Area Type: Other
 Comment: Problem 7

(N-S) Johnson St.
 File Name: Exerc_07.HC9
 8-5-96

Traffic and Roadway Conditions

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	1	2	<	1	2	<	1	2	<	1	2	<
Volumes	50	250	75	75	500	25	100	1250	50	125	750	50
PHF or PK15	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85	0.85
Lane Width	11.0	11.0		11.0	11.0		11.0	11.0		11.0	11.0	
Grade		0			0			0			0	
% Heavy Veh	5	5	5	5	5	5	5	5	5	5	5	5
Parking	(Y/N)	N		(Y/N)	N		(Y/N)	N		(Y/N)	N	
Bus Stops			0			0			0			0
Con. Peds			100			100			100			100
Ped Button	(Y/N)	Y	20.0 s	(Y/N)	Y	20.0 s	(Y/N)	Y	20.0 s	(Y/N)	Y	20.0 s
Arr Type	3	3		3	3		3	3		3	3	
RTOR Vols			0			0			0			0
Lost Time	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00

Signal Operations

Phase Combination	1	2	3	4	5	6	7	8
EB Left	*				NB Left	*		
Thru					Thru		*	
Right		*			Right		*	
Peds		*			Peds	*		
WB Left		*			SB Left	*		
Thru			*		Thru		*	
Right			*		Right		*	
Peds			*		Peds	*		
NB Right					EB Right			
SB Right					WB Right			
Green		7.0P	19.0P		Green	18.0P	45.0P	
Yellow/AR		4.0	4.0		Yellow/AR	4.0	4.0	

Cycle Length: 105 sec Phase combination order: #1 #2 #5 #6

Volume Adjustment Worksheet

Dir	Lane	Lane	Adj
-----	------	------	-----

tion/ Mvt	Mvt Vol	PHF	Adj Vol	Lane Grp	Grp Vol	No. Ln	Util Fact	Growth Fact	Grp Vol	Prop LT	Prop RT
EB											
Left	50	0.85	59	L	59	1	1.000	1.000	59	1.00	0.00
Thru	250	0.85	294	TR	382	2	1.050	1.000	401	0.00	0.23
Right	75	0.85	88								
WB											
Left	75	0.85	88	L	88	1	1.000	1.000	88	1.00	0.00
Thru	500	0.85	588	TR	617	2	1.050	1.000	648	0.00	0.05
Right	25	0.85	29								
NB											
Left	100	0.85	118	L	118	1	1.000	1.000	118	1.00	0.00
Thru	1250	0.85	1471	TR	1530	2	1.050	1.000	1606	0.00	0.04
Right	50	0.85	59								
SB											
Left	125	0.85	147	L	147	1	1.000	1.000	147	1.00	0.00
Thru	750	0.85	882	TR	941	2	1.050	1.000	988	0.00	0.06
Right	50	0.85	59								

Saturation Flow Adjustment Worksheet

Direction /LnGrp	Ideal Sat Flow	No. Lns	f W	f HV	f G	f p	f BB	f A	f RT	f LT	Adj Sat Flow
EB											
L	1900	1	0.97	0.95	1.00	1.00	1.00	1.00	1.00	0.95	1662
TR	1900	2	0.97	0.95	1.00	1.00	1.00	1.00	0.95	1.00	3339
WB											
L	1900	1	0.97	0.95	1.00	1.00	1.00	1.00	1.00	0.95	1662
TR	1900	2	0.97	0.95	1.00	1.00	1.00	1.00	0.99	1.00	3466
NB											
L	1900	1	0.97	0.95	1.00	1.00	1.00	1.00	1.00	0.95	1662
TR	1900	2	0.97	0.95	1.00	1.00	1.00	1.00	0.99	1.00	3478
SB											
L	1900	1	0.97	0.95	1.00	1.00	1.00	1.00	1.00	0.95	1662
TR	1900	2	0.97	0.95	1.00	1.00	1.00	1.00	0.99	1.00	3466

Capacity Analysis Worksheet

Direction /LnGrp	Adj Flow Rate (v)	Adj Sat Flow Rate (s)	Flow Ratio (v/s)	Green Ratio (g/C)	Lane Group Capacity (c)	v/c Ratio
EB						
L	59	1662	0.035	0.076	127	0.466
TR	401	3339	0.120	0.190	636	0.631
WB						
L	88	1662	0.053	0.076	127	0.695 *
TR	648	3466	0.187	0.190	660	0.982 *
NB						
L	118	1662	0.071	0.181	301	0.392
TR	1606	3478	0.462	0.438	1524	1.054 *

SB
 L 147 1662 0.088 0.181 301 0.489 *
 TR 988 3466 0.285 0.438 1518 0.651
 Sum (v/s) critical = 0.790
 Lost Time/Cycle, L = 12.0 sec **Critical v/c(x) = 0.892**

 Level of Service Worksheet

Direction /LnGrp	v/c Ratio	g/C Ratio	Delay d 1	Del Adj Fact	Lane Group Cap	Calib d 2	Delay d 2	Lane Grp Del	Lane Grp LOS	Delay By App	LOS By App
EB											
L	0.466	0.076	35.3	1.000	127	16	2.0	37.3	D	31.9	D
TR	0.631	0.190	29.7	1.000	636	16	1.4	31.2	D		
WB											
L	0.695	0.076	36.0	1.000	127	16	10.0	46.0	E	53.9	E
TR	0.982	0.190	32.2	1.000	660	16	22.8	55.0	E		
NB											
L	0.392	0.181	28.8	1.000	301	16	0.5	29.3	D	53.7	E
TR	1.054	0.438	22.4	1.000	1524	16	33.1	55.5	E		
SB											
L	0.489	0.181	29.4	1.000	301	16	1.0	30.4	D	19.9	C
TR	0.651	0.438	17.6	1.000	1518	16	0.7	18.3	C		
			Intersection Delay = 41.8 sec/veh Intersection LOS = E								

9.8

The following is the Highway Capacity Software (HCS) solution to the exercise. This is one suitable intersection layout and phasing sequence that provides a condition of under capacity.

HIGHWAY CAPACITY MANUAL SIGNALIZED INTERSECTION PLANNING METHOD
 LANE VOLUME WORKSHEET

File name: Date: 8 - 2 - 19 96 Time Period:

(E/W): (N/S): Analyst :

Peak hour factor: .9 Comment: Exercise 08

EAST WEST NORTH SOUTH
 BOUND BOUND BOUND BOUND

LEFT TURN MOVEMENT

1. LT volume	100	125	150	200
2. Opposing mainline volume	645	700	1200	1150
3. Number of exclusive LT lanes	0	0	1	1
Cross Product [2] * [1]	64500	87500	180000	230000
Left Lane Configuration (E=Excl, S=Shrd):	S	S	E	E
Left Turn Treatment Type:	Perm	Perm	Perm	Perm
4. LT adjustment factor	N/A	N/A	1.0	1.0
5. LT lane vol	N/A	N/A	N/A	N/A

RIGHT TURN MOVEMENT

Right Lane Configuration (E=Excl, S=Shrd)	S	S	S	S
6. RT volume	100	120	250	200
7. Exclusive lanes	N/A	N/A	N/A	N/A
8. RT adjustment factor	.85	.85	.85	.85
9. Exclusive RT lane volume	0	0	0	0
10. Shared lane vol	118	141	294	235

THROUGH MOVEMENT

11. Thru volume	500	400	900	1000
12. Parking adjustment factor	1	1	1	1
13. No. of thru lanes including shared	2	2	2	2
14. Total approach volume	618	541	1194	1235
15. Prop. of left turns in lane group	N/A	N/A	0	0
16. Left turn equivalence	4.14	4.8	10.4	9.35
17. LT adj. factor:	.63	.57	N/A	N/A
18. Through lane volume	494	477	597	618
19. Critical lane volume	494	477	597	618

Left Turn Check (if [16] > 8)

20. Permitted left turn sneaker capacity: 7200/Cmax	N/A	N/A	60	60
---	-----	-----	----	----

HIGHWAY CAPACITY MANUAL SIGNALIZED INTERSECTION PLANNING METHOD
SIGNAL OPERATIONS WORKSHEET

File name: Date: 8 - 2 - 19 96 Time Period:

(E/W):

(N/S):

Analyst :

EAST BOUND	WEST BOUND	NORTH BOUND	SOUTH BOUND
---------------	---------------	----------------	----------------

Phase Plan Selection from Lane Volume Worksheet

Critical through-RT vol: [19]	494	477	597	618
LT lane vol: [5]	N/A	N/A	N/A	N/A
Left turn protection: (P/U/N)	U	U	U	U
Dominant left turn: (Indicate by '*')		*		*

Selection Criteria based on the
specified left turn protection

Plan 1:	U	U	U	U
Plan 2a:	U	P	U	P
Plan 2b:	P	U	P	U
Plan 3a:	*P	P	*P	P
Plan 3b:	P	*P	P	*P
Plan 4:	N	N	N	N

* Indicates the dominant left turn
for each opposing pair

Min. cycle (Cmin)	60	Max. cycle (Cmax)	120				
Timing Plan				--- EAST-WEST ---	----	-- NORTH-SOUTH ---	---
	Value	Ph 1	Ph 2	Ph 3	Ph 1	Ph 2	Ph 3
Movement codes		EWG			NSG		
Critical phase vol [CV]		494	0	0	618	0	0
Critical sum [CS]	1112						
CBD adjustment [CBD]	.9						
Reference sum [RS]	1385						
Lost time/phase [PL]		3	0	0	3	0	0
Lost time/cycle [TL]	6						
Cycle length [CYC]	60						
Green time		27	0	0	33	0	0
Critical v/c ratio [Xcm]	0.76						
Status		Under capacity.					

9.9

Refer to the solution to Exercise 4. A critical V/c ratio of 0.90 or lower is not achieved. The overall intersection LOS is D.

9.10

a.) 2-phase signal, pretimed

E-W flow ratio, $V/s = 0.50$

N-S flow ratio, $V/s = 0.25$

Lost time = change interval = 4 sec/phase (8 sec/cycle)

The minimum cycle length is computed using $X_c = 1.00$

$$C_{\min} = \frac{LX_c}{X_c - \sum(V/s)_{ci}} = \frac{8(1.0)}{1.0 - (0.5 + 0.25)} = 32 \text{ sec}$$

If a V/c of no more than 0.8 were desired, then $X = 0.8$

$$C = \frac{8(0.8)}{0.8 - (0.5 + 0.25)} = 128 \text{ sec (cycle too long)}$$

Try $X = 0.85$

$$C = \frac{8(0.85)}{0.85 - (0.5 + 0.25)} = 68 \text{ sec} \approx 70 \text{ sec (rounding to nearest 5 sec)}$$

The actual V/c ratio provided by the 70-sec cycle is

$$X_c = \sum_i \left(\frac{V}{s} \right)_{ci} \frac{C}{C - L} = (0.5 + 0.25) \left(\frac{70}{70 - 8} \right) = 0.85$$

1.) If the greens are allocated so that the V/c ratios for critical movements in each phase are equal, then

$$g_i = \left(\frac{V}{s} \right)_i \left(\frac{C}{X_i} \right)$$

$$g_1 = 0.5 \left(\frac{70}{0.85} \right) = 41 \text{ sec}$$

$$g_2 = 0.25 \left(\frac{70}{0.85} \right) = 21 \text{ sec}$$

Total = 62 sec

2.) If the minimum green time is assigned to the minor approach, and the balance to the major approach, then

$$g_2 = 0.25 \left(\frac{70}{1.0} \right) = 17.5 \text{ sec}$$

$$g_1 = 70 - 8 - 17.5 = 44.5 \text{ sec}$$

Total = 62 sec

b.) 2-phase semi-actuated signal

E-W flow ratio, $V/s = 0.65$

N-S flow ratio, $V/s = 0.10$

Designed to operate at critical $V/c = 0.85$

Lost time = 9 sec/cycle

$$C = \frac{9(0.85)}{0.85 - (0.65 + 0.1)} = 76.5 \text{ sec}$$

Because the signal is semi-actuated, the cycle length is not rounded off.

The green time for the side street is estimated using $V/c = 1.0$

$$g_2 = 0.1 \left(\frac{76.5}{1.0} \right) = 7.65 \text{ sec}$$

$$g_1 = 76.5 - 9 - 7.65 = 59.85 \text{ sec}$$

9.11

The following is the Highway Capacity Software (HCS) solution to the exercise. The specific answers requested in the problem statement are highlighted in bold.

Center For Microcomputers In Transportation

HCS: Unsignalized Intersection Release 2.1 Page 1

File Name Exerc_11.HC0

Streets: (N-S) Molalla Ave. (E-W) Main St.

Analyst.....

Date of Analysis..... 7/19/96

Other Information.....

All-way Stop-controlled Intersection

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	0>	2<	0	0>	2<	0	0>	1<	0	0>	1<	0
Volumes	50	325	75	50	225	75	45	245	60	35	225	50
PHF	.95	.95	.95	.95	.95	.95	.95	.95	.95	.95	.95	.95
Grade		0			0			0			0	
MC's*(%)	0	0	0	0	0	0	0	0	0	0	0	0
SU/RV's (%)	0	0	0	0	0	0	0	0	0	0	0	0
CV's*(%)	0	0	0	0	0	0	0	0	0	0	0	0
PCE's	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1	1.1

*MC's Motor Cycles CV's Combination Vehicles

Volume Summary and Capacity Analysis Worksheet

EB WB NB SB

LT Flow Rate	53	53	47	37
RT Flow Rate	79	79	63	53
Approach Flow Rate	474	369	368	327
Proportion LT	0.11	0.14	0.13	0.11
Proportion RT	0.17	0.21	0.17	0.16
Opposing Approach Flow Rate	369	474	327	368
Conflicting Approaches Flow Rate	695	695	843	843
Proportion, Subject Approach Flow Rate	0.31	0.24	0.24	0.21
Proportion, Opposing Approach Flow Rate	0.24	0.31	0.21	0.24
Lanes on Subject Approach	2	2	1	1
Lanes on Opposing Approach	2	2	1	1
LT, Opposing Approach	53	53	37	47
RT, Opposing Approach	79	79	53	63
LT, Conflicting Approaches	84	84	106	106
RT, Conflicting Approaches	116	116	158	158
Proportion LT, Opposing Approach	0.14	0.11	0.11	0.13
Proportion RT, Opposing Approach	0.21	0.17	0.16	0.17
Proportion LT, Conflicting Approaches	0.12	0.12	0.13	0.13
Proportion RT, Conflicting Approaches	0.17	0.17	0.19	0.19
Approach Capacity	690	669	505	495

Intersection Performance Summary

Movement	Approach Flow Rate	Approach Capacity	V/C Ratio	Average Total Delay	LOS
EB	474	690	0.69	13.6	C
WB	369	669	0.55	8.1	B
NB	368	505	0.73	15.9	C
SB	327	495	0.66	12.3	C

Intersection Delay = 12.58
Level of Service (Intersection) = C

9.12

The following is the Highway Capacity Software (HCS) solution to the exercise. The specific answers requested in the problem statement are highlighted in bold.

HCS: Signalized Intersection Version 2.4a 08-07-1996 1

=====
Streets: (E-W) Moss Avenue (N-S) Ryder Street
Analyst: File Name: Exerc_12.HC9
Area Type: Other 8-7-96
=====

Traffic and Roadway Conditions

Eastbound			Westbound			Northbound			Southbound		
L	T	R	L	T	R	L	T	R	L	T	R
-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----

No. Lanes	1	1	1	1	1	1	1	1	1	1	1	1
Volumes	105	450	105	125	500	80	100	560	90	95	525	40
PHF or PK15	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Lane Width	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0
Grade		0			0			0			0	
% Heavy Veh	3	3	3	3	3	3	5	5	5	5	5	5
Parking	(Y/N)	N		(Y/N)	N		(Y/N)	N		(Y/N)	N	
Bus Stops			0			0			0			0
Con. Peds			0			0			0			0
Ped Button	(Y/N)	N		(Y/N)	N		(Y/N)	N		(Y/N)	N	
Arr Type	3	3	3	3	3	3	5	5	5	5	5	5
RTOR Vols			0			0			0			0
Lost Time	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00

Signal Operations

Phase Combination	1	2	3	4		5	6	7	8
EB Left	*				NB Left	*			
Thru	*				Thru	*			
Right	*				Right	*			
Peds					Peds				
WB Left	*				SB Left	*			
Thru	*				Thru	*			
Right	*				Right	*			
Peds					Peds				
NB Right					EB Right				
SB Right					WB Right				
Green	46.0P				Green	46.0P			
Yellow/AR	4.0				Yellow/AR	4.0			

Cycle Length: 100 sec Phase combination order: #1 #5
=====

Volume Adjustment Worksheet

Direction/ Mvt	Mvt Vol	PHF	Adj Vol	Lane Grp	Lane No. Vol	Lane Util Ln	Growth Fact	Adj Grp Vol	Prop LT	Prop RT
EB										
Left	105	0.90	117	L	117	1	1.000	1.000	117	1.00 0.00
Thru	450	0.90	500	T	500	1	1.000	1.000	500	0.00 0.00
Right	105	0.90	117	R	117	1	1.000	1.000	117	0.00 1.00
WB										
Left	125	0.90	139	L	139	1	1.000	1.000	139	1.00 0.00
Thru	500	0.90	556	T	556	1	1.000	1.000	556	0.00 0.00
Right	80	0.90	89	R	89	1	1.000	1.000	89	0.00 1.00
NB										
Left	100	0.90	111	L	111	1	1.000	1.000	111	1.00 0.00
Thru	560	0.90	622	T	622	1	1.000	1.000	622	0.00 0.00
Right	90	0.90	100	R	100	1	1.000	1.000	100	0.00 1.00
SB										

Left	95	0.90	106	L	106	1	1.000	1.000	106	1.00	0.00
Thru	525	0.90	583	T	583	1	1.000	1.000	583	0.00	0.00
Right	40	0.90	44	R	44	1	1.000	1.000	44	0.00	1.00

Saturation Flow Adjustment Worksheet

Direction /LnGrp	Ideal Sat Flow	No. Lns	f W	f HV	f G	f p	f BB	f A	f RT	f LT	Adj Sat Flow
EB											
L	1900	1	1.00	0.97	1.00	1.00	1.00	1.00	1.00	0.13	244
T	1900	1	1.00	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1845
R	1900	1	1.00	0.97	1.00	1.00	1.00	1.00	0.85	1.00	1568
WB											
L	1900	1	1.00	0.97	1.00	1.00	1.00	1.00	1.00	0.17	318
T	1900	1	1.00	0.97	1.00	1.00	1.00	1.00	1.00	1.00	1845
R	1900	1	1.00	0.97	1.00	1.00	1.00	1.00	0.85	1.00	1568
NB											
L	1900	1	1.00	0.95	1.00	1.00	1.00	1.00	1.00	0.16	297
T	1900	1	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1810
R	1900	1	1.00	0.95	1.00	1.00	1.00	1.00	0.85	1.00	1538
SB											
L	1900	1	1.00	0.95	1.00	1.00	1.00	1.00	1.00	0.13	234
T	1900	1	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1810
R	1900	1	1.00	0.95	1.00	1.00	1.00	1.00	0.85	1.00	1538

Supplemental Permitted LT Worksheet

APPROACH	EB
Cycle Length, C	100
Actual Green Time for Lane Group, G	46
Effective Green Time for Lane Group, g	47
Opposing Effective Green Time, go	47
Number of Opposing Lanes, No	1
Number of Lanes in Lane Group, N	1
Adjusted Left-Turn Flow Rate, Vlt	117
Proportion of Left Turns in Lane Group, Plt	1.00
Left Turns per Cycle: LTC=Vlt*C/3600	3.25
Adjusted Opposing Flow Rate, Vo	556
Opposing Flow per Lane, Per Cycle: Volc=VoC/3600No	15.44
Opposing Platoon Ratio, Rpo	1
Lost time per phase, t1	3
gf=Gexp(-0.882*LTC^0.717)-t1	0.00
Opposing Queue Ratio: qro=1-Rpo(go/C)	0.53
gq = Volc * qro / (.5 - Volc * (1 - qro) / go)-t1	20.69
gu=g-gq (or g-gf)	26.31
fs=(875-0.625Vo)/1000	0.53
Pl=Plt[1+{(N-1)g/(fsgu+4.5)}]	1.00
El1	4.24
fmin	0.09
fm, (min=fmin;max=1.00)	0.13
flt=[fm+0.91(N-1)]/N	0.13

APPROACH

WB

Cycle Length, C	100
Actual Green Time for Lane Group, G	46
Effective Green Time for Lane Group, g	47
Opposing Effective Green Time, go	47
Number of Opposing Lanes, No	1
Number of Lanes in Lane Group, N	1
Adjusted Left-Turn Flow Rate, Vlt	139
Proportion of Left Turns in Lane Group, Plt	1.00
Left Turns per Cycle: LTC=Vlt*C/3600	3.86
Adjusted Opposing Flow Rate, Vo	500
Opposing Flow per Lane, Per Cycle: Volc=VoC/3600No	13.89
Opposing Platoon Ratio, Rpo	1
Lost time per phase, tl	3
gf=Gexp(-0.882*LTC^0.717)-tl	0.00
Opposing Queue Ratio: qro=1-Rpo(go/C)	0.53
gq = Volc * qro / (.5 - Volc * (1 - qro) / go)-tl	17.38
gu=g-gq (or g-gf)	29.62
fs=(875-0.625Vo)/1000	0.56
Pl=Plt[1+{(N-1)g/(fsgu+4.5)}]	1.00
El1	3.65
fmin	0.09
fm, (min=fmin;max=1.00)	0.17
flt=[fm+0.91(N-1)]/N	0.17

APPROACH	NB
Cycle Length, C	100
Actual Green Time for Lane Group, G	46
Effective Green Time for Lane Group, g	47
Opposing Effective Green Time, go	47
Number of Opposing Lanes, No	1
Number of Lanes in Lane Group, N	1
Adjusted Left-Turn Flow Rate, Vlt	111
Proportion of Left Turns in Lane Group, Plt	1.00
Left Turns per Cycle: LTC=Vlt*C/3600	3.08
Adjusted Opposing Flow Rate, Vo	583
Opposing Flow per Lane, Per Cycle: Volc=VoC/3600No	16.19
Opposing Platoon Ratio, Rpo	1.67
Lost time per phase, tl	3
gf=Gexp(-0.882*LTC^0.717)-tl	0.00
Opposing Queue Ratio: qro=1-Rpo(go/C)	0.22
gq = Volc * qro / (.5 - Volc * (1 - qro) / go)-tl	12.17
gu=g-gq (or g-gf)	34.83
fs=(875-0.625Vo)/1000	0.51
Pl=Plt[1+{(N-1)g/(fsgu+4.5)}]	1.00
El1	4.52
fmin	0.09
fm, (min=fmin;max=1.00)	0.16
flt=[fm+0.91(N-1)]/N	0.16

APPROACH	SB
Cycle Length, C	100
Actual Green Time for Lane Group, G	46
Effective Green Time for Lane Group, g	47
Opposing Effective Green Time, go	47
Number of Opposing Lanes, No	1
Number of Lanes in Lane Group, N	1

Adjusted Left-Turn Flow Rate, Vlt 106
 Proportion of Left Turns in Lane Group, Plt 1.00
 Left Turns per Cycle: LTC=Vlt*C/3600 2.94
 Adjusted Opposing Flow Rate, Vo 622
 Opposing Flow per Lane, Per Cycle: Volc=VoC/3600No 17.28
 Opposing Platoon Ratio, Rpo 1.67
 Lost time per phase, t1 3
 $gf=Gexp(-0.882*LTC^{0.717})-t1$ 0.00
 Opposing Queue Ratio: $qro=1-Rpo(go/C)$ 0.22
 $gq = Volc * qro / (.5 - Volc * (1 - qro) / go)-t1$ 14.58
 $gu=g-gq$ (or $g-gf$) 32.42
 $fs=(875-0.625Vo)/1000$ 0.49
 $Pl=Plt[1+\{(N-1)g/(fsgu+4.5)\}]$ 1.00
 Ell 5.33
 fmin 0.09
 $fm, (min=fmin;max=1.00)$ 0.13
 $flt=[fm+0.91(N-1)]/N$ 0.13

=====
 Capacity Analysis Worksheet
 =====

Direction /LnGrp	Adj Flow Rate (v)	Adj Sat Flow Rate (s)	Flow Ratio (v/s)	Green Ratio (g/C)	Lane Group Capacity (c)	v/c Ratio
EB						
L	117	244	0.480	0.470	115	1.020 *
T	500	1845	0.271	0.470	867	0.577
R	117	1568	0.075	0.470	737	0.159
WB						
L	139	318	0.437	0.470	149	0.930
T	556	1845	0.301	0.470	867	0.641
R	89	1568	0.057	0.470	737	0.121
NB						
L	111	297	0.374	0.470	140	0.795
T	622	1810	0.344	0.470	851	0.731
R	100	1538	0.065	0.470	723	0.138
SB						
L	106	234	0.453	0.470	110	0.964 *
T	583	1810	0.322	0.470	851	0.685
R	44	1538	0.029	0.470	723	0.061

Sum (v/s) critical = 0.932

Lost Time/Cycle, L = 6.0 sec **Critical v/c(x) = 0.992**

=====
 Level of Service Worksheet
 =====

Direction /LnGrp	v/c Ratio	g/C Ratio	Delay d 1	Del Adj Fact	Lane Group Cap	Calib d 2	Delay d 2	Lane Grp Del	Lane Grp LOS	Delay By App	LOS By App
EB											
L	1.020	0.470	20.1	1.000	115	16	71.7	91.8	F	26.9	D
T	0.577	0.470	14.6	1.000	867	16	0.7	15.4	C		
R	0.159	0.470	11.5	1.000	737	16	0.0	11.5	B		
WB											
L	0.930	0.470	19.0	1.000	149	16	37.9	56.9	E	23.0	C
T	0.641	0.470	15.3	1.000	867	16	1.1	16.4	C		

R	0.121	0.470	11.3	1.000	737	16	0.0	11.3	B		
NB											
L	0.795	0.470	17.0	0.409	140	8	10.0	16.9	C	8.6	B
T	0.731	0.470	16.3	0.409	851	8	1.2	7.8	B		
R	0.138	0.470	11.4	0.409	723	8	0.0	4.7	A		
SB											
L	0.964	0.470	19.5	0.409	110	8	37.1	45.1	E	12.6	B
T	0.685	0.470	15.7	0.409	851	8	0.8	7.3	B		
R	0.061	0.470	11.0	0.409	723	8	0.0	4.5	A		
					Intersection Delay = 17.6 sec/veh Intersection LOS = C						

9.13

The following is the Highway Capacity Software (HCS) solution to the exercise. The specific answers requested in the problem statement are highlighted in bold.

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HCS:  Signalized Intersection      Version 2.4a                08-07-1996  1
=====
Streets: (E-W) Moss Avenue          (N-S) Ryder Street
Analyst:                            File Name: Exerc_13.HC9
Area Type: Other                    8-7-96
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Traffic and Roadway Conditions

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	1	1	1	1	1	1	1	1	1	1	1	1
Volumes	105	450	105	125	500	80	100	560	90	95	525	40
PHF or PK15	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90	0.90
Lane Width	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0	12.0
Grade		0			0			0			0	
% Heavy Veh	3	3	3	3	3	3	5	5	5	5	5	5
Parking	(Y/N)	N		(Y/N)	N		(Y/N)	N		(Y/N)	N	
Bus Stops			0			0			0			0
Con. Peds			0			0			0			0
Ped Button	(Y/N)	N		(Y/N)	N		(Y/N)	N		(Y/N)	N	
Arr Type	3	3	3	3	3	3	5	5	5	5	5	5
RTOR Vols			0			0			0			0
Lost Time	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00

Signal Operations

Phase Combination	1	2	3	4	5	6	7	8
EB Left	*				NB Left	*		
Thru		*			Thru		*	
Right		*			Right		*	
Peds					Peds			
WB Left		*			SB Left	*		
Thru			*		Thru		*	
Right			*		Right		*	

R	1900	1	1.00	0.95	1.00	1.00	1.00	1.00	1.00	0.85	1.00	1538
SB												
L	1900	1	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	0.95	1719
T	1900	1	1.00	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1810
R	1900	1	1.00	0.95	1.00	1.00	1.00	1.00	1.00	0.85	1.00	1538

Capacity Analysis Worksheet

Direction /LnGrp	Adj Flow Rate (v)	Adj Sat Flow Rate (s)	Flow Ratio (v/s)	Green Ratio (g/C)	Lane Group Capacity (c)	v/c Ratio
EB						
L	117	1752	0.067	0.080	140	0.835
T	500	1845	0.271	0.330	609	0.821
R	117	1568	0.075	0.440	690	0.170
WB						
L	139	1752	0.079	0.080	140	0.992 *
T	556	1845	0.301	0.330	609	0.913 *
R	89	1568	0.057	0.440	690	0.129
NB						
L	111	1719	0.065	0.080	138	0.807 *
T	622	1810	0.344	0.390	706	0.881 *
R	100	1538	0.065	0.500	769	0.130
SB						
L	106	1719	0.062	0.080	138	0.771
T	583	1810	0.322	0.390	706	0.826
R	44	1538	0.029	0.500	769	0.057
Sum (v/s) critical = 0.789						
Lost Time/Cycle, L = 12.0 sec Critical v/c(x) = 0.896						

Level of Service Worksheet

Direction /LnGrp	v/c Ratio	g/C Ratio	Delay d 1	Del Adj Fact	Lane Group Cap	Calib d 2	Delay d 2	Lane Grp Del	Lane Grp LOS	Delay By App	LOS By App
EB											
L	0.835	0.080	34.5	1.000	140	16	22.3	56.8	E	31.2	D
T	0.821	0.330	23.4	1.000	609	16	6.1	29.5	D		
R	0.170	0.440	12.9	1.000	690	16	0.0	12.9	B		
WB											
L	0.992	0.080	34.9	1.000	140	16	55.9	90.8	F	44.1	E
T	0.913	0.330	24.4	1.000	609	16	13.1	37.5	D		
R	0.129	0.440	12.6	1.000	690	16	0.0	12.6	B		
NB											
L	0.807	0.080	34.4	0.942	138	8	11.0	43.3	E	19.1	C
T	0.881	0.390	21.5	0.574	706	8	4.9	17.2	C		
R	0.130	0.500	10.2	0.333	769	8	0.0	3.4	A		
SB											
L	0.771	0.080	34.3	0.942	138	8	8.5	40.8	E	18.0	C

```

T      0.826 0.390 20.9 0.574 706 8 3.0 14.9 B
R      0.057 0.500 9.8 0.333 769 8 0.0 3.3 A
Intersection Delay = 28.1 sec/veh Intersection LOS = D

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9.14

The following is the Highway Capacity Software (HCS) solution to the exercise. The specific answers requested in the problem statement are highlighted in bold.

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HCS: Signalized Intersection Version 2.4a 08-07-1996 1

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Streets: (E-W) 5th Avenue (N-S) James Street
Analyst: File Name: Exerc_14.HC9
Area Type: Other 8-7-96

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Traffic and Roadway Conditions

	Eastbound			Westbound			Northbound			Southbound		
	L	T	R	L	T	R	L	T	R	L	T	R
No. Lanes	1	2	<	1	2	1	1	1	<	1	1	1
Volumes	175	980	75	140	1020	205	75	245	25	50	160	95
PHF or PK15	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Lane Width	11.0	11.0		11.0	11.0	11.0	11.0	11.0		10.0	10.0	10.0
Grade		2			-2			0			0	
% Heavy Veh	2	2	2	2	2	2	5	5	5	5	5	5
Parking	(Y/N)	N		(Y/N)	N		(Y/N)	Y	40	(Y/N)	Y	25
Bus Stops			4			2			0			0
Con. Peds			50			50			50			50
Ped Button	(Y/N)	Y	15.0 s	(Y/N)	Y	15.0 s	(Y/N)	Y	20.0 s	(Y/N)	Y	20.0
Arr Type	4	4		4	4	4	3	3		3	3	3
RTOR Vols			0			0			0			0
Lost Time	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00	3.00

Signal Operations

Phase Combination	1	2	3	4	5	6	7	8
EB Left	*				NB Left	*		
Thru		*			Thru	*		
Right			*		Right	*		
Peds			*		Peds	*		
WB Left		*			SB Left	*		
Thru			*		Thru	*		
Right			*		Right	*		
Peds			*		Peds	*		
NB Right					EB Right			
SB Right					WB Right			
Green	10.0P	44.0P			Green	24.0P		
Yellow/AR	4.0	4.0			Yellow/AR	4.0		

Cycle Length: 90 sec Phase combination order: #1 #2 #5

Volume Adjustment Worksheet

Direction/ Mvt	Mvt Vol	PHF	Adj Vol	Lane Grp	Lane Grp Vol	No. Ln	Lane Util Fact	Growth Fact	Adj Grp Vol	Prop LT	Prop RT
EB											
Left	175	0.95	184	L	184	1	1.000	1.000	184	1.00	0.00
Thru	980	0.95	1032	TR	1111	2	1.050	1.000	1167	0.00	0.07
Right	75	0.95	79								
WB											
Left	140	0.95	147	L	147	1	1.000	1.000	147	1.00	0.00
Thru	1020	0.95	1074	T	1074	2	1.050	1.000	1128	0.00	0.00
Right	205	0.95	216	R	216	1	1.000	1.000	216	0.00	1.00
NB											
Left	75	0.95	79	L	79	1	1.000	1.000	79	1.00	0.00
Thru	245	0.95	258	TR	284	1	1.000	1.000	284	0.00	0.09
Right	25	0.95	26								
SB											
Left	50	0.95	53	L	53	1	1.000	1.000	53	1.00	0.00
Thru	160	0.95	168	T	168	1	1.000	1.000	168	0.00	0.00
Right	95	0.95	100	R	100	1	1.000	1.000	100	0.00	1.00

Saturation Flow Adjustment Worksheet

Direction /LnGrp	Ideal Sat Flow	No. Lns	f W	f HV	f G	f p	f BB	f A	f RT	f LT	Adj Sat Flow
EB											
L	1900	1	0.97	0.98	0.99	1.00	1.00	1.00	1.00	0.95	1694
TR	1900	2	0.97	0.98	0.99	1.00	0.99	1.00	0.99	1.00	3494
WB											
L	1900	1	0.97	0.98	1.01	1.00	1.00	1.00	1.00	0.95	1728
T	1900	2	0.97	0.98	1.01	1.00	1.00	1.00	1.00	1.00	3637
R	1900	1	0.97	0.98	1.01	1.00	1.00	1.00	0.83	1.00	1497
NB											
L	1900	1	0.97	0.95	1.00	1.00	1.00	1.00	1.00	0.51	887
TR	1900	1	0.97	0.95	1.00	0.70	1.00	1.00	0.98	1.00	1205
SB											
L	1900	1	0.93	0.95	1.00	1.00	1.00	1.00	1.00	0.26	440
T	1900	1	0.93	0.95	1.00	1.00	1.00	1.00	1.00	1.00	1689
R	1900	1	0.93	0.95	1.00	0.77	1.00	1.00	0.83	1.00	1081

Supplemental Permitted LT Worksheet

APPROACH

NB

Cycle Length, C	90
Actual Green Time for Lane Group, G	24
Effective Green Time for Lane Group, g	25
Opposing Effective Green Time, go	25
Number of Opposing Lanes, No	1
Number of Lanes in Lane Group, N	1
Adjusted Left-Turn Flow Rate, Vlt	79
Proportion of Left Turns in Lane Group, Plt	1.00
Left Turns per Cycle: LTC=Vlt*C/3600	1.98
Adjusted Opposing Flow Rate, Vo	168
Opposing Flow per Lane, Per Cycle: Volc=VoC/3600No	4.20
Opposing Platoon Ratio, Rpo	1
Lost time per phase, tl	3
gf=Gexp(-0.882*LTC^0.717)-tl	0.00
Opposing Queue Ratio: qro=1-Rpo(go/C)	0.72
gq = Volc * qro / (.5 - Volc * (1 - qro) / go)-tl	3.69
gu=g-gq (or g-gf)	21.31
fs=(875-0.625Vo)/1000	0.77
Pl=Plt[1+{(N-1)g/(fsgu+4.5)}]	1.00
E11	1.68
fmin	0.16
fm, (min=fmin;max=1.00)	0.51
flt=[fm+0.91(N-1)]/N	0.51

APPROACH	SB
Cycle Length, C	90
Actual Green Time for Lane Group, G	24
Effective Green Time for Lane Group, g	25
Opposing Effective Green Time, go	25
Number of Opposing Lanes, No	1
Number of Lanes in Lane Group, N	1
Adjusted Left-Turn Flow Rate, Vlt	53
Proportion of Left Turns in Lane Group, Plt	1.00
Left Turns per Cycle: LTC=Vlt*C/3600	1.33
Adjusted Opposing Flow Rate, Vo	284
Opposing Flow per Lane, Per Cycle: Volc=VoC/3600No	7.10
Opposing Platoon Ratio, Rpo	1
Lost time per phase, tl	3
gf=Gexp(-0.882*LTC^0.717)-tl	0.00
Opposing Queue Ratio: qro=1-Rpo(go/C)	0.72
gq = Volc * qro / (.5 - Volc * (1 - qro) / go)-tl	9.18
gu=g-gq (or g-gf)	15.82
fs=(875-0.625Vo)/1000	0.70
Pl=Plt[1+{(N-1)g/(fsgu+4.5)}]	1.00
E11	2.43
fmin	0.16
fm, (min=fmin;max=1.00)	0.26
flt=[fm+0.91(N-1)]/N	0.26

=====

Capacity Analysis Worksheet

Direction /LnGrp	Adj Flow Rate (v)	Adj Sat Flow Rate (s)	Flow Ratio (v/s)	Green Ratio (g/C)	Lane Group Capacity (c)	v/c Ratio
EB						
L	184	1694	0.109	0.122	207	0.889 *
TR	1167	3494	0.334	0.500	1747	0.668 *
WB						
L	147	1728	0.085	0.122	211	0.696
T	1128	3637	0.310	0.500	1818	0.620
R	216	1497	0.144	0.500	748	0.289
NB						
L	79	887	0.089	0.278	246	0.321
TR	284	1205	0.236	0.278	335	0.848 *
SB						
L	53	440	0.120	0.278	122	0.434
T	168	1689	0.099	0.278	469	0.358
R	100	1081	0.093	0.278	300	0.333
				Sum (v/s) critical =	0.678	
Lost Time/Cycle, L =		9.0 sec	Critical v/c(x)		=	0.754

Level of Service Worksheet

Direction /LnGrp	v/c Ratio	g/C Ratio	Delay d 1	Del Adj Fact	Lane Group Cap	Calib d 2	Delay d 2	Lane Grp Del	Lane Grp LOS	Delay By App	LOS By App
EB											
L	0.889	0.122	29.6	1.000	207	12	19.3	48.9	E	15.6	C
TR	0.668	0.500	12.8	0.767	1747	12	0.5	10.4	B		
WB											
L	0.696	0.122	28.8	1.000	211	12	5.0	33.8	D	11.9	B
T	0.620	0.500	12.4	0.767	1818	12	0.4	9.9	B		
R	0.289	0.500	10.0	0.767	748	12	0.0	7.7	B		
NB											
L	0.321	0.278	19.6	1.000	246	16	0.3	19.9	C	32.4	D
TR	0.848	0.278	23.3	1.000	335	16	12.5	35.9	D		
SB											
L	0.434	0.278	20.3	1.000	122	16	1.6	21.8	C	20.3	C
T	0.358	0.278	19.8	1.000	469	16	0.2	20.0	C		
R	0.333	0.278	19.7	1.000	300	16	0.3	19.9	C		
Intersection Delay =						16.2 sec/veh	Intersection LOS = C				